

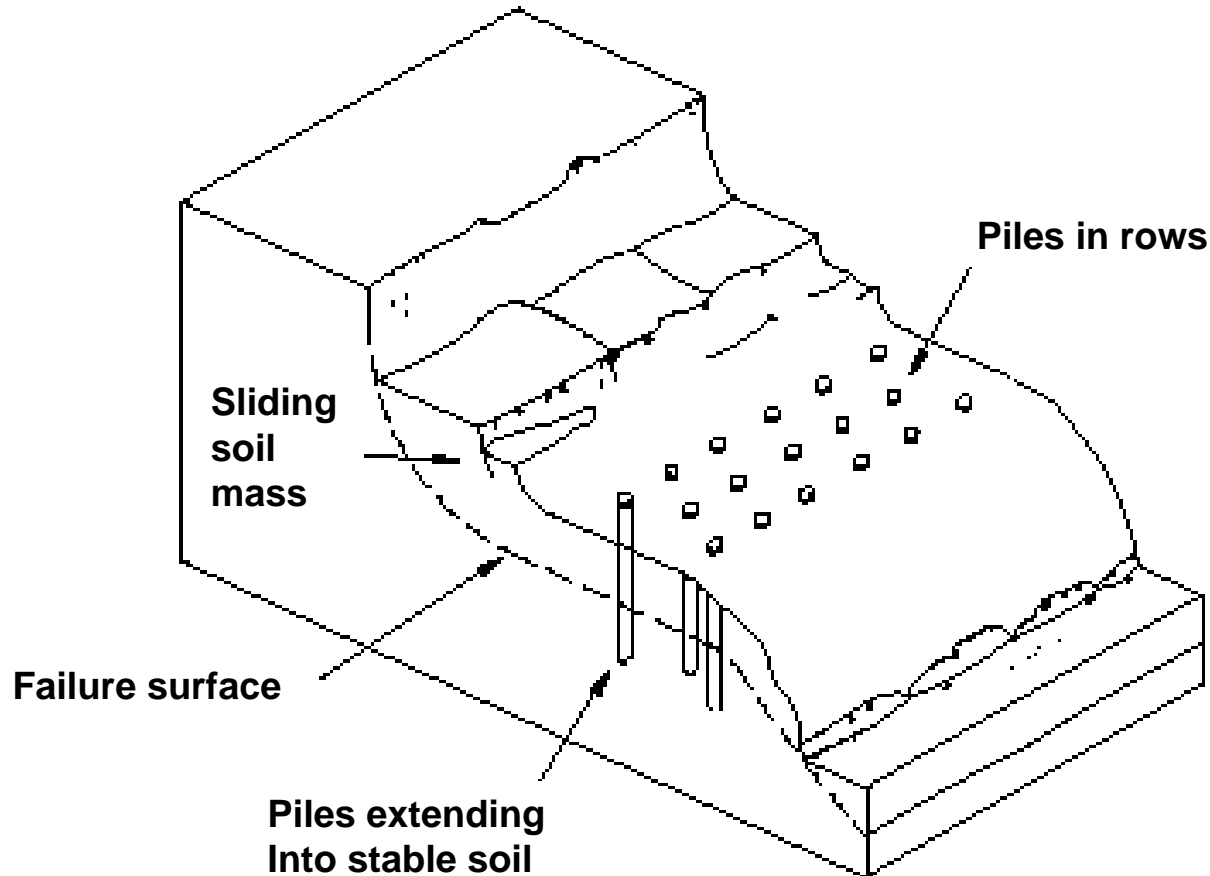
# **SLOPE STABILIZATION USING DRIVEN PILES**

**Mohamed Ashour, Ph.D., P.E.  
University of Alabama, Huntsville**

**Southeastern Transportation Geotechnical  
Engineering Conference (STGEC)**

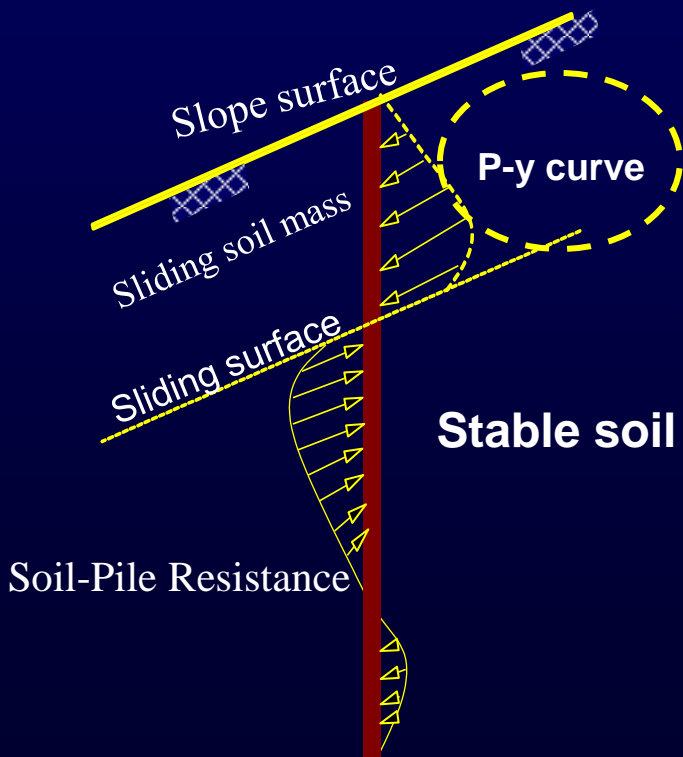
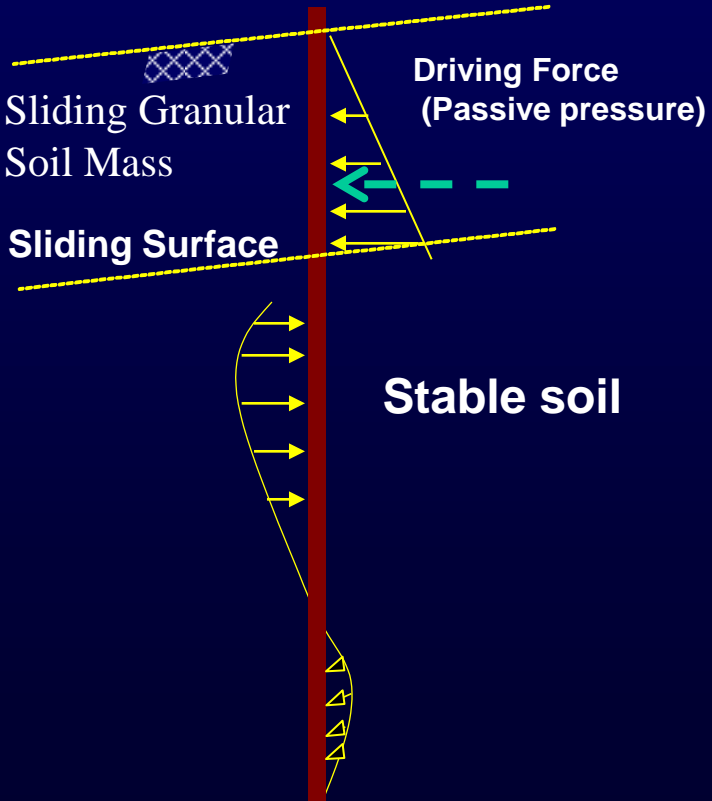
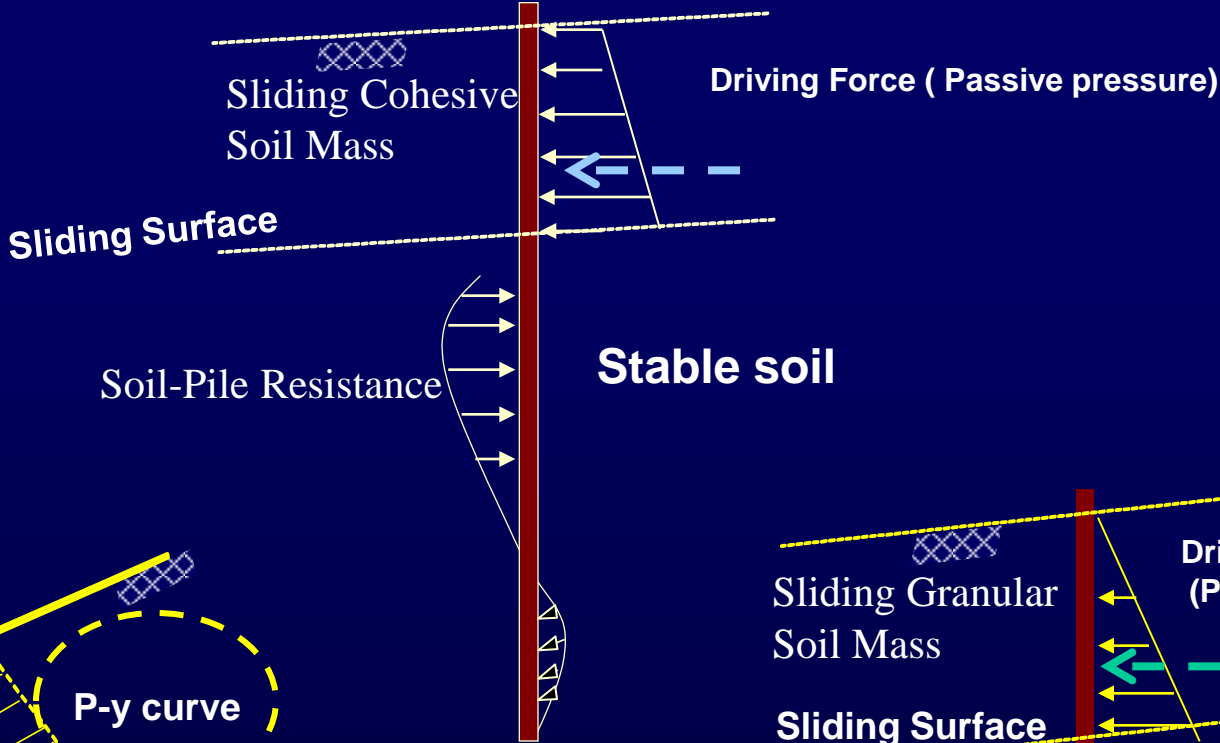
**October 5, 2010**

# Slope Stabilizing Piles/Shfts Effectively Act as Shear Dowels across the Slip Plane



Pile Rows for Slope Stabilization (Thomson et al. 2005)

# Current Practice

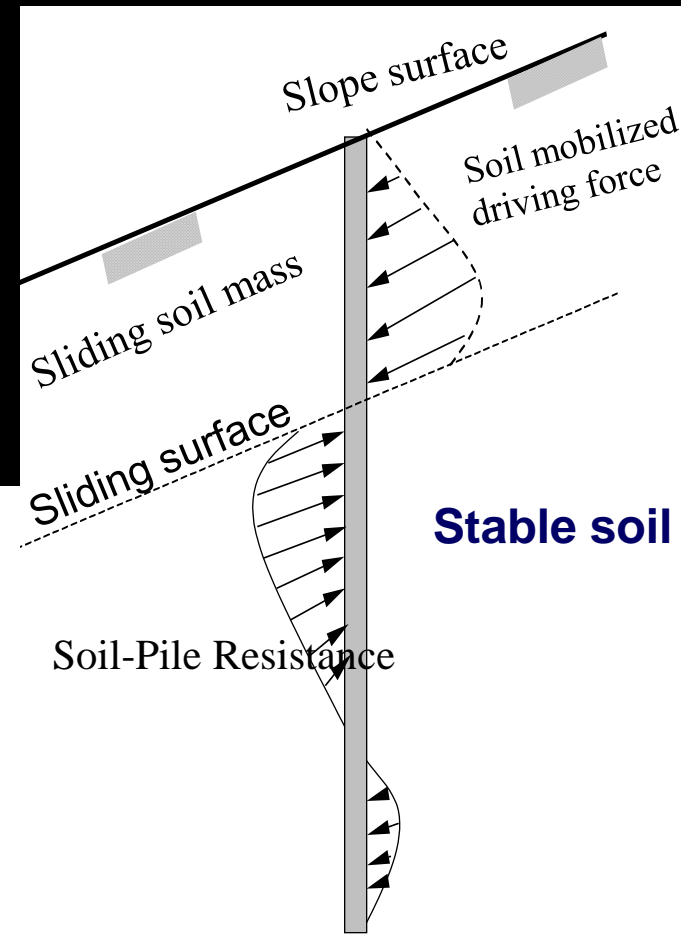
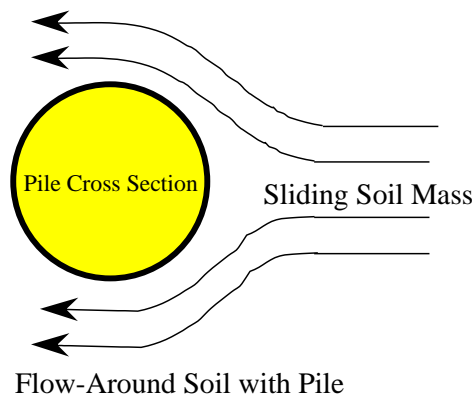


# CHALLENGES:

- **Characterization and Evaluation of the Mobilized Lateral Pressure Induced by the Moving Soil Mass on the Pile**

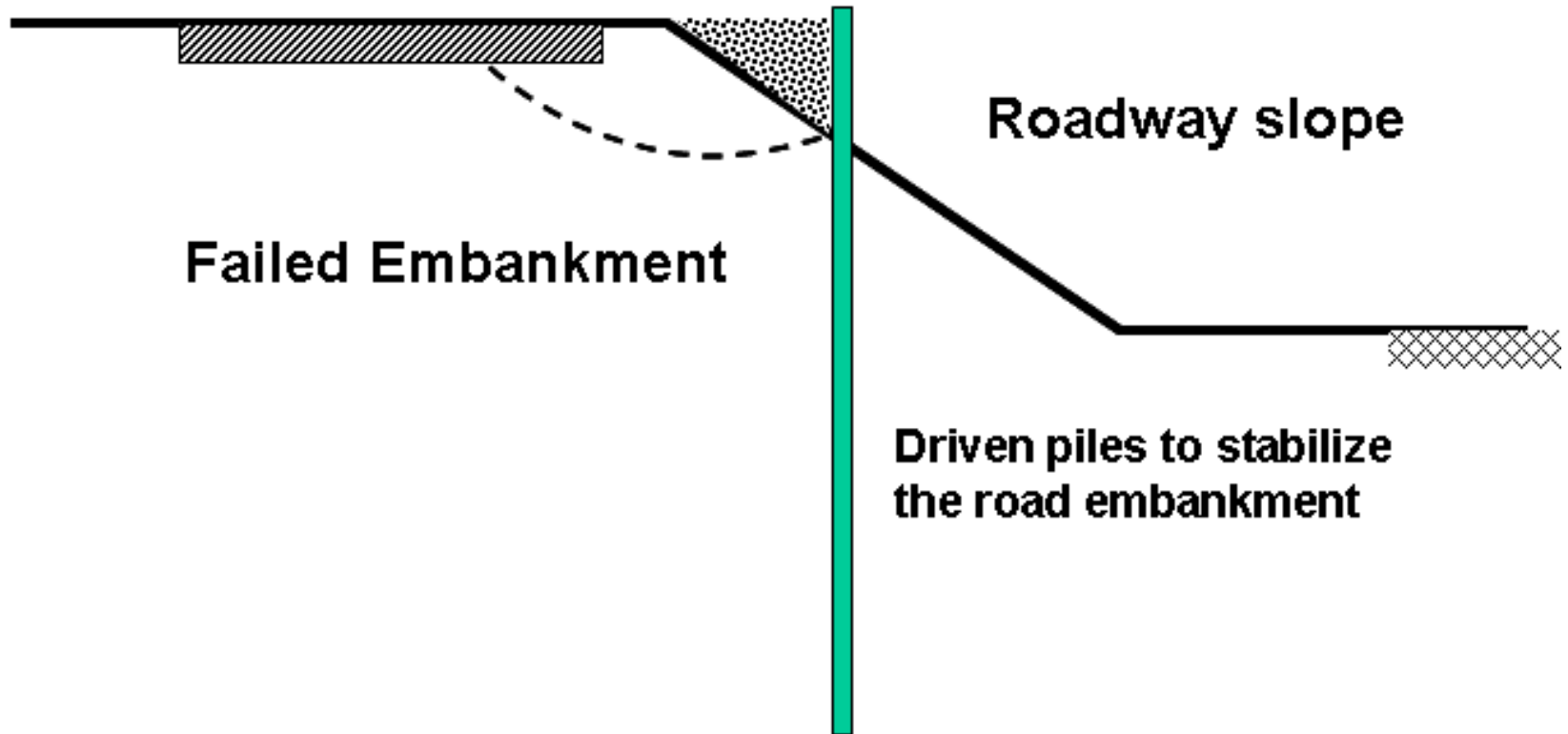
- **Interaction between Stabilizing Piles and Soil Arching Effect**

- **Soil Flow-around Failure**

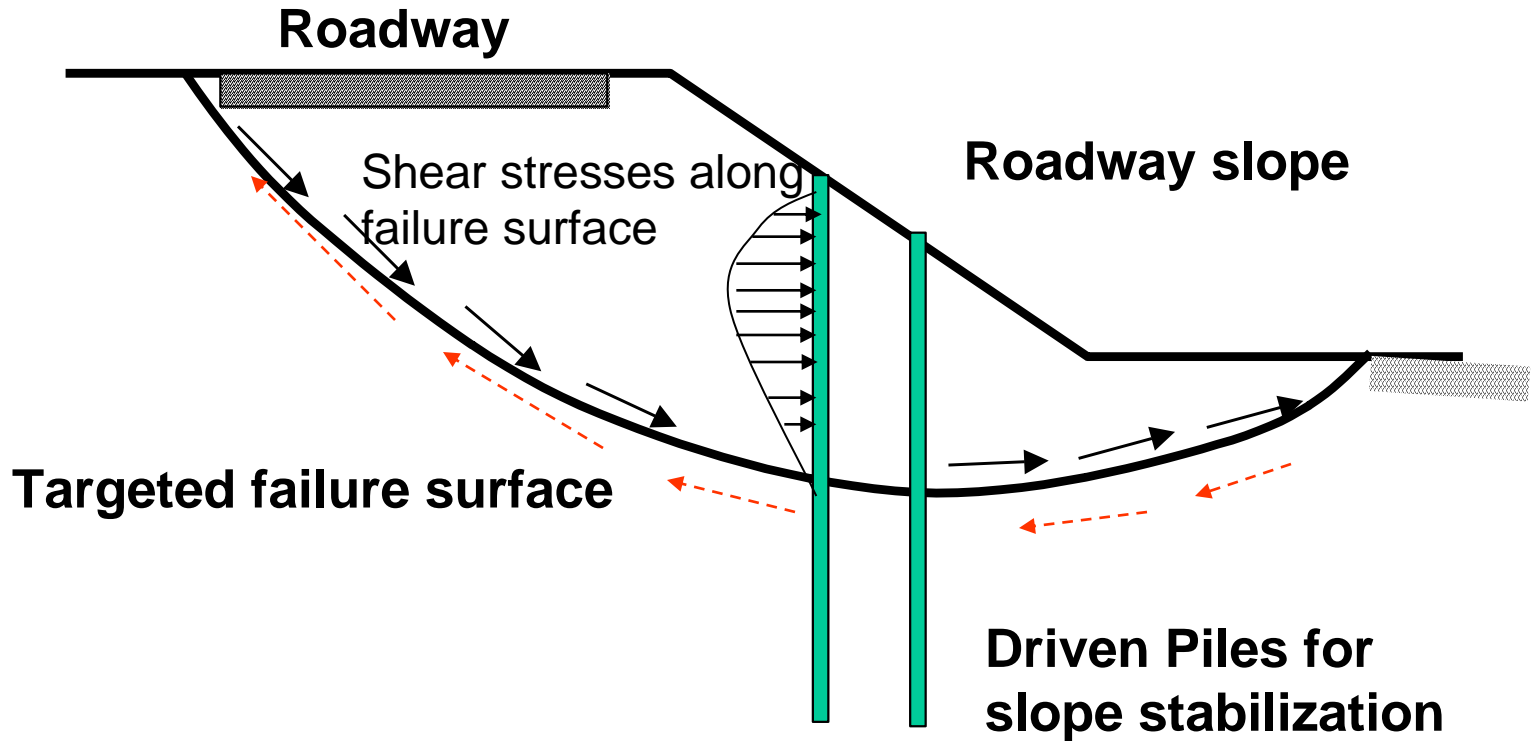




## B. Pre-existing Failure Surface



# C. Anticipated Failure Surface

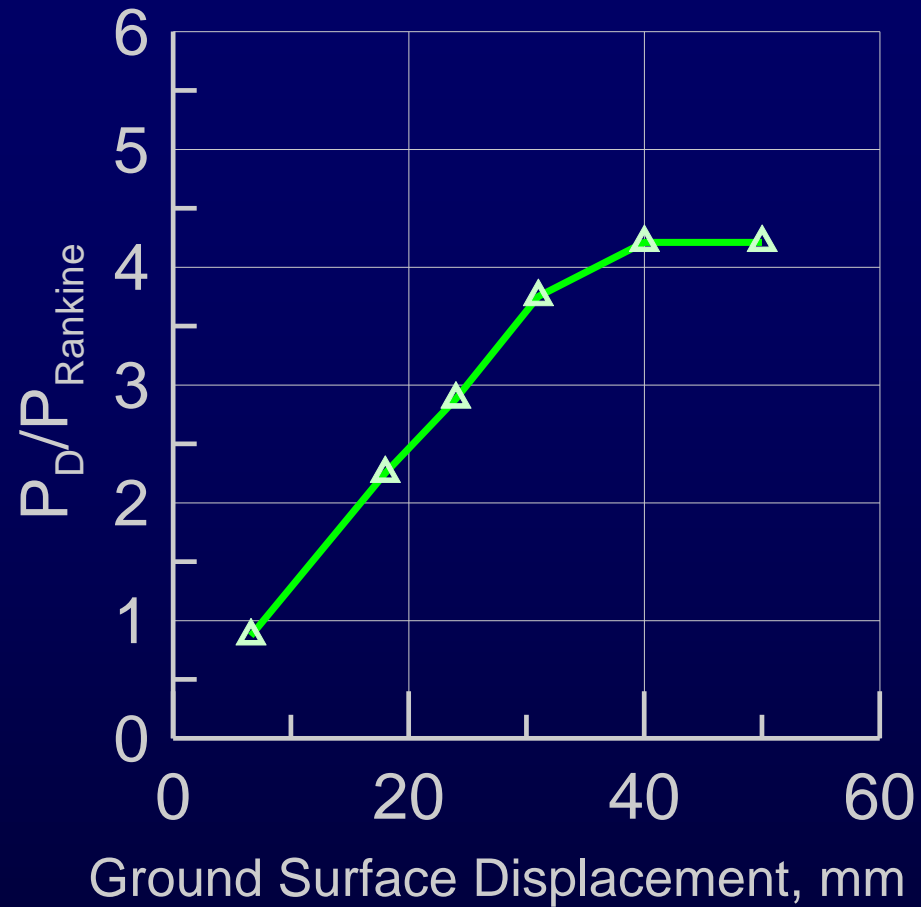
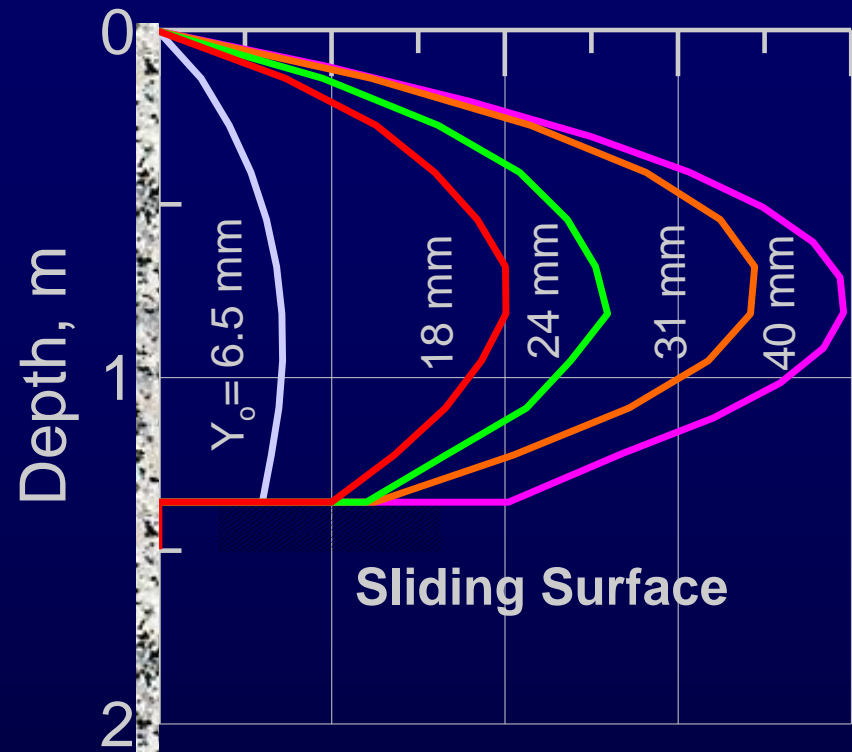






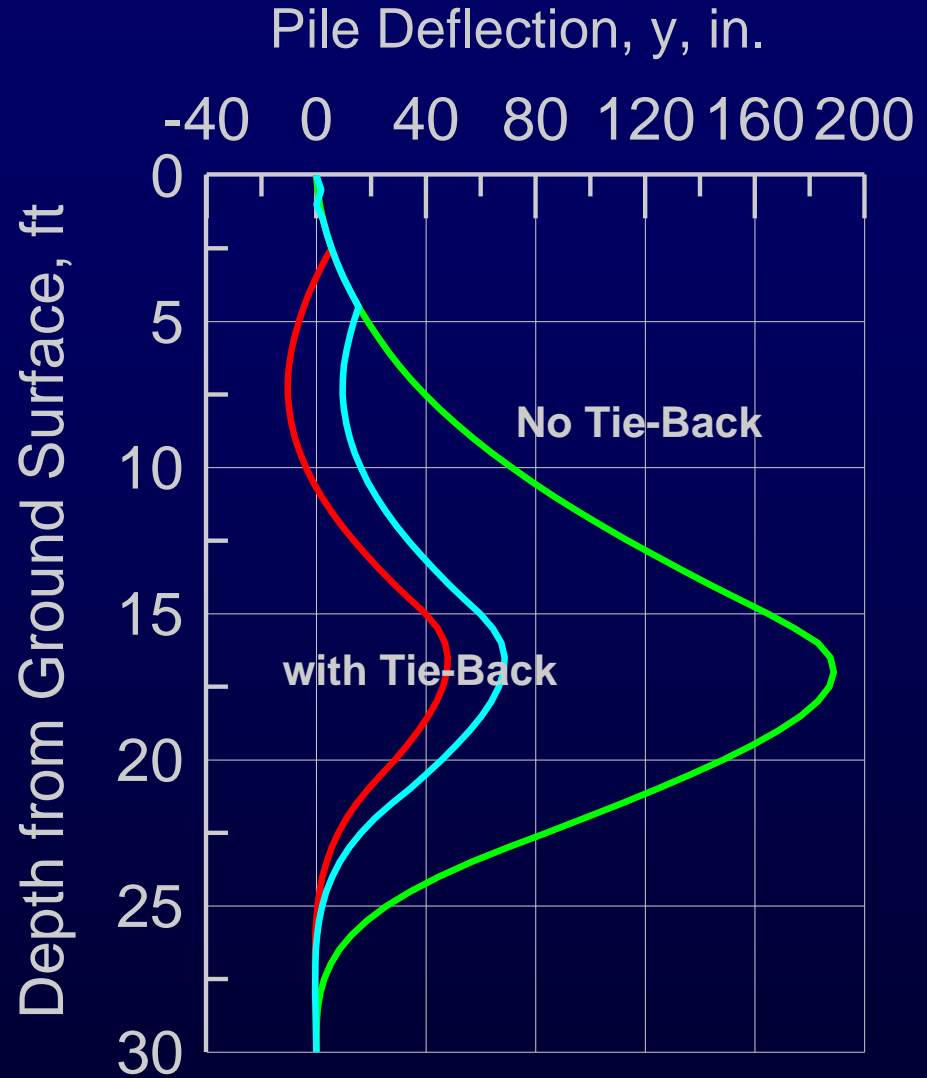
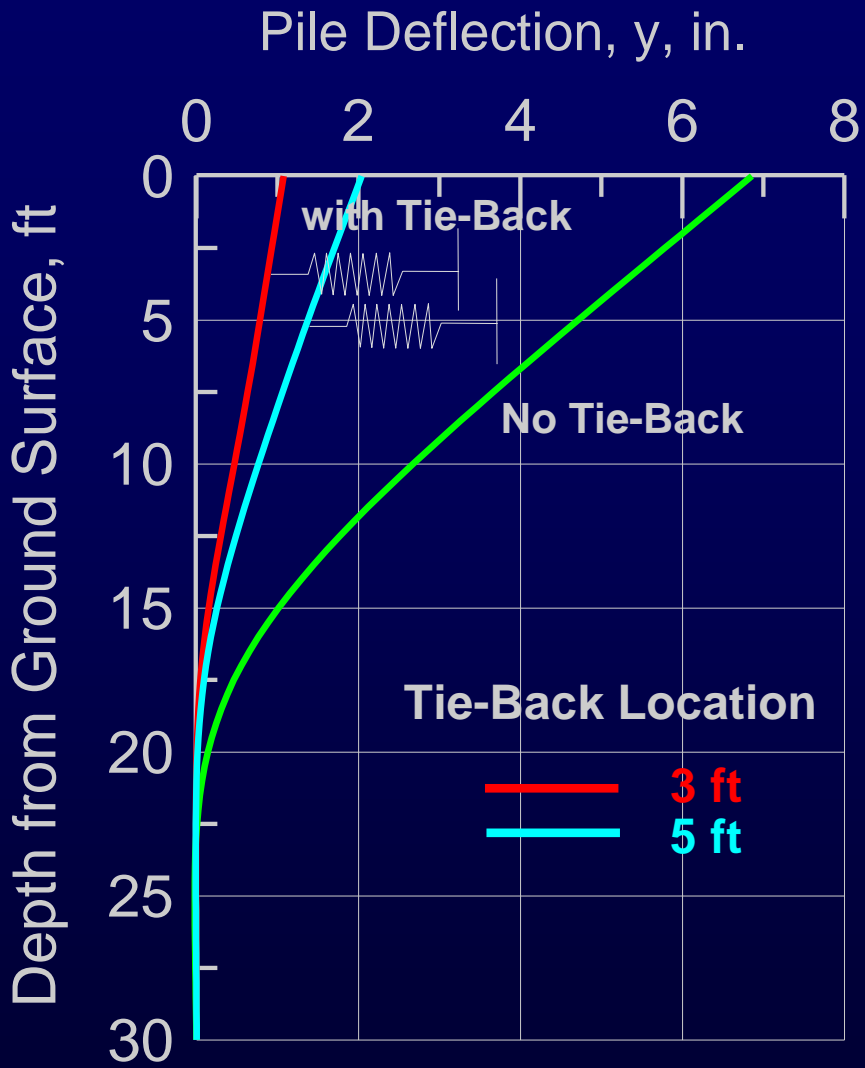
Soil Driving Pressure,  $p_D$ , kN/m

0 20 40 60 80



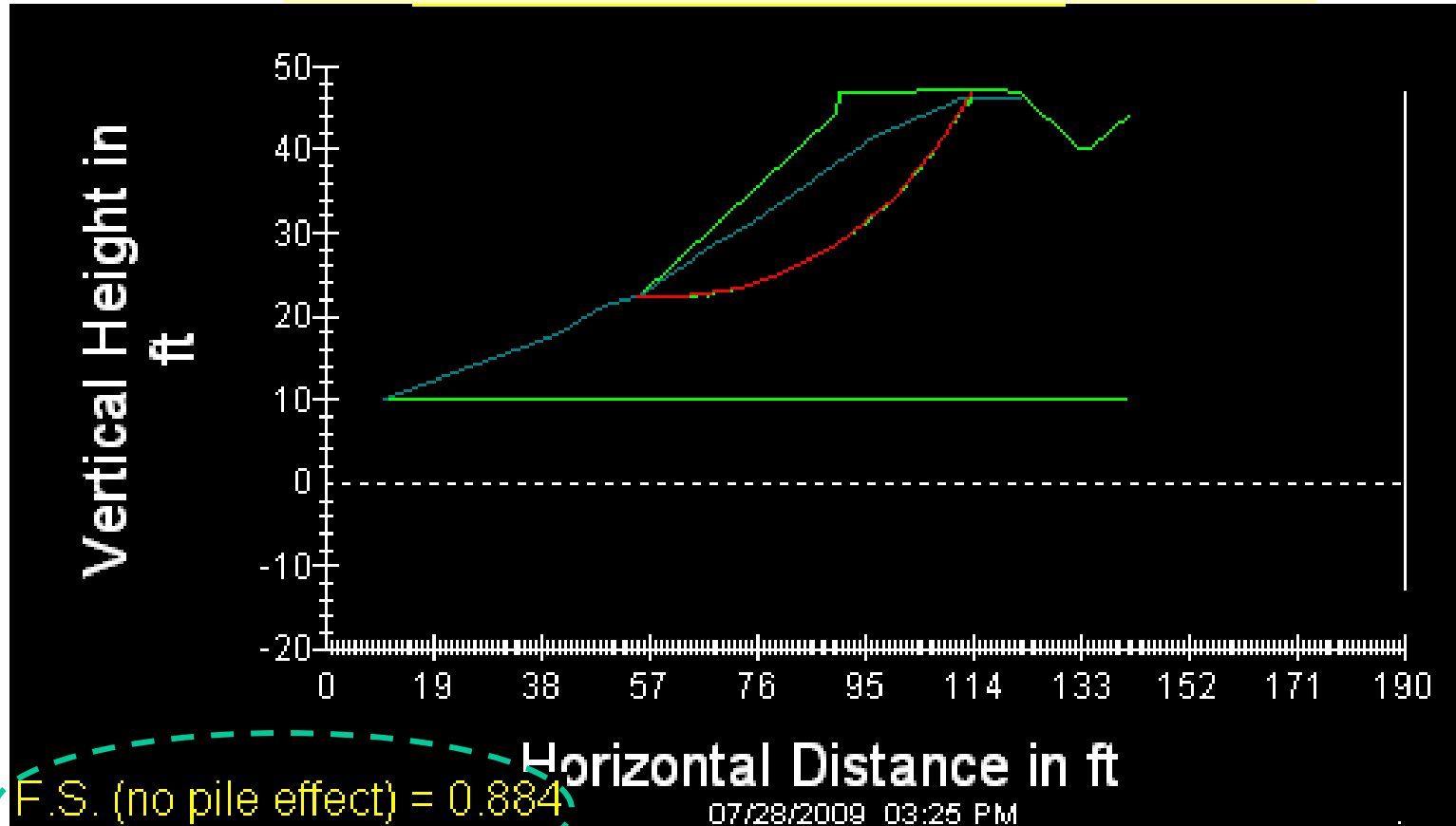
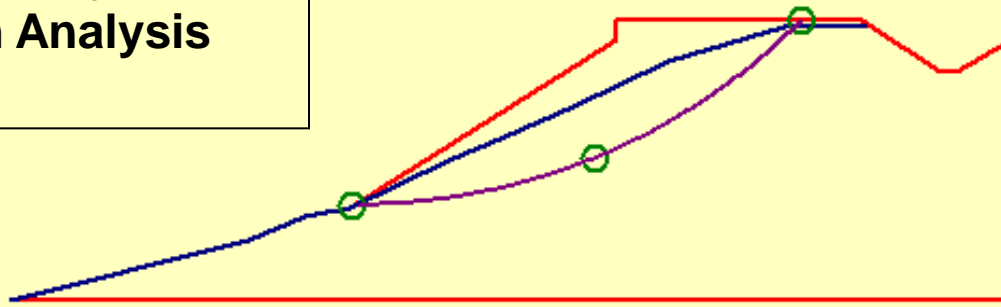
**Progressive Driving Force Induced  
by Sliding Soil Mass**

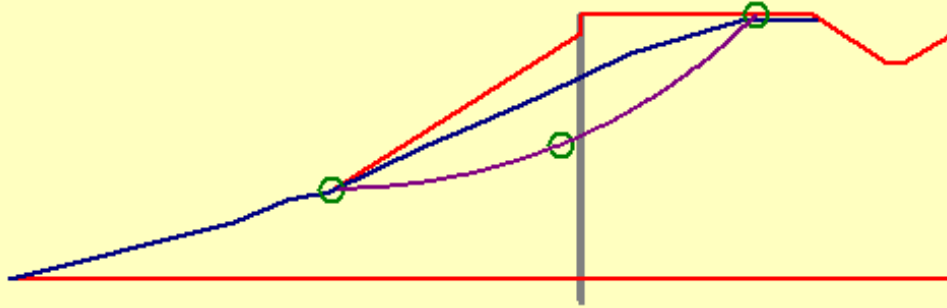
# TIE-BACK IN PSSLOPE



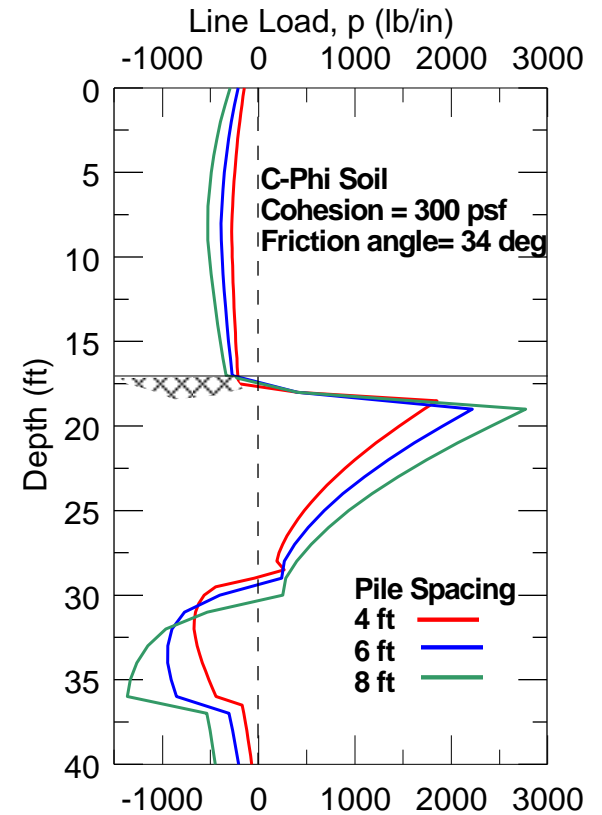
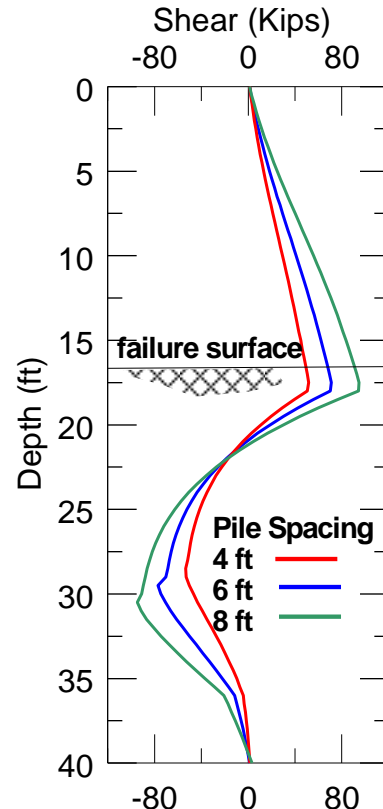
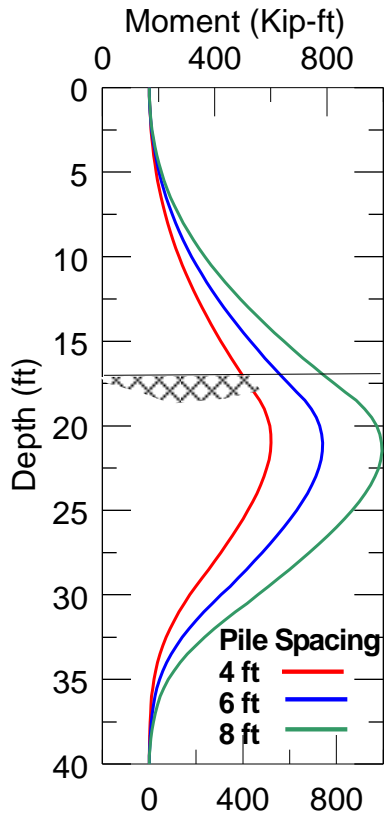
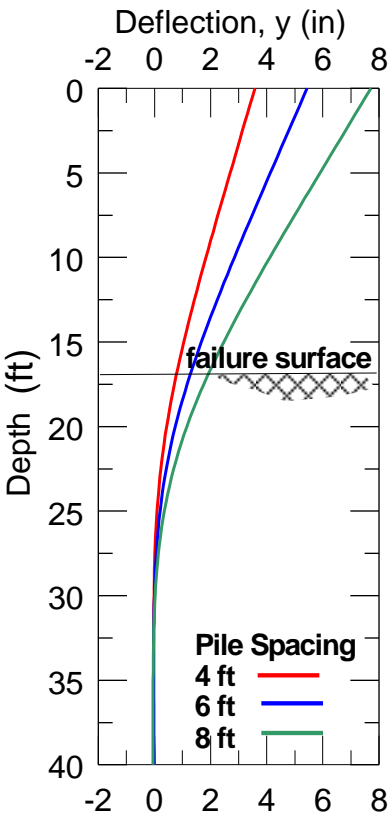
| Depth from Pile Head | Horizontal Spacing | Vertical Inclination Angle | Tie-Back Length | Tie-Back Section        | Factored Resistance |
|----------------------|--------------------|----------------------------|-----------------|-------------------------|---------------------|
| (ft)                 | (ft)               | with Horiz. (deg)          | ft              | Area (in <sup>2</sup> ) | (Kips)              |
| 3                    | 4                  | 0                          | 30              | 3                       | 10                  |

# Slope Stability Limit Equilibrium Analysis



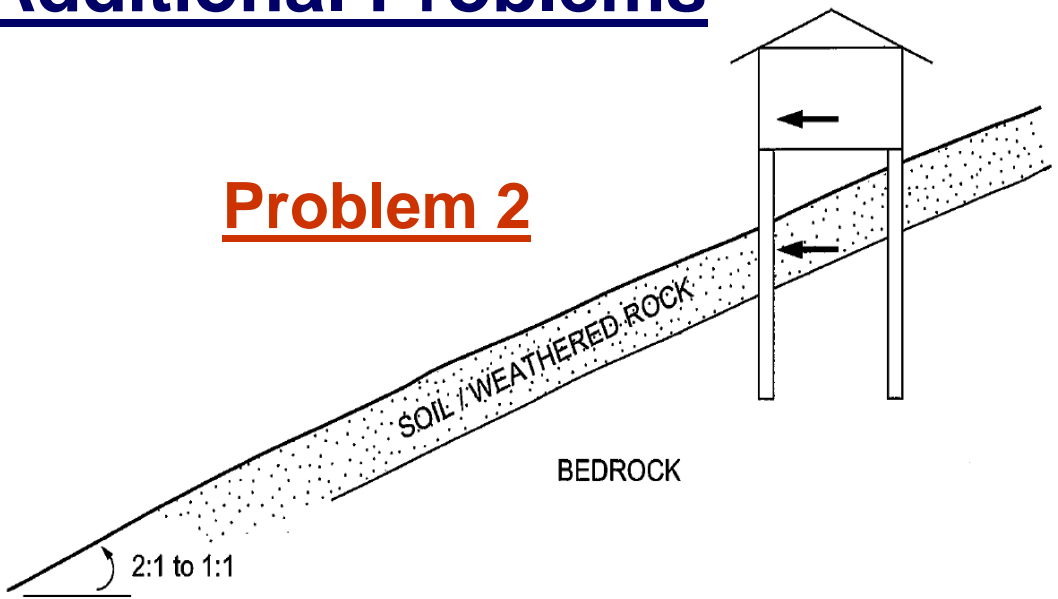


**Pile**  
**W 14 x 211**  
 **$M_p = 1625$  kip-ft**  
**Desired FS of Supported Slope = 1.3**

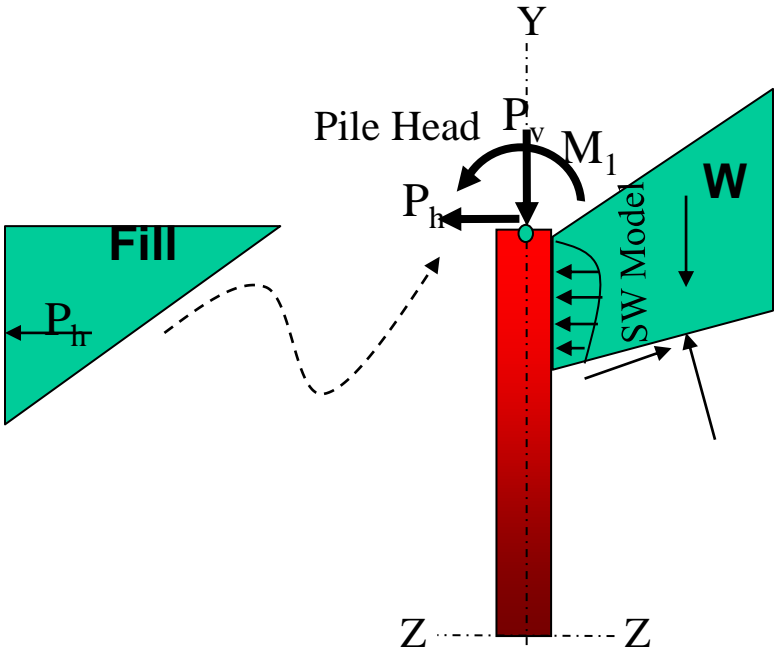


# Additional Problems

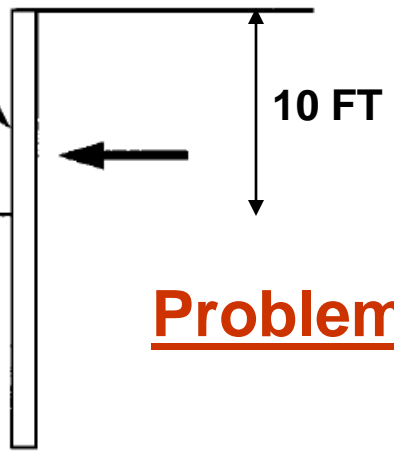
## Problem 2



Bedrock is stable, but soil/weathered rock exerts a lateral load on piles.

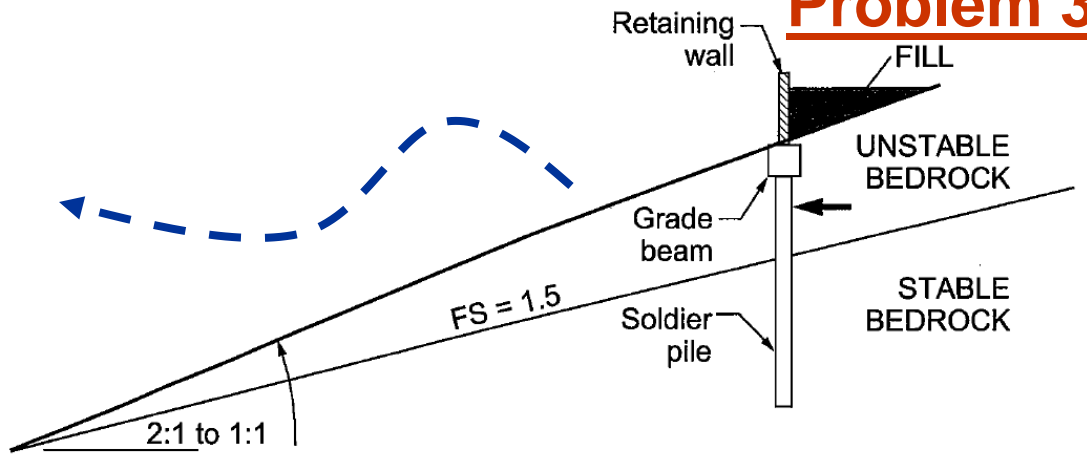


Shoring pile



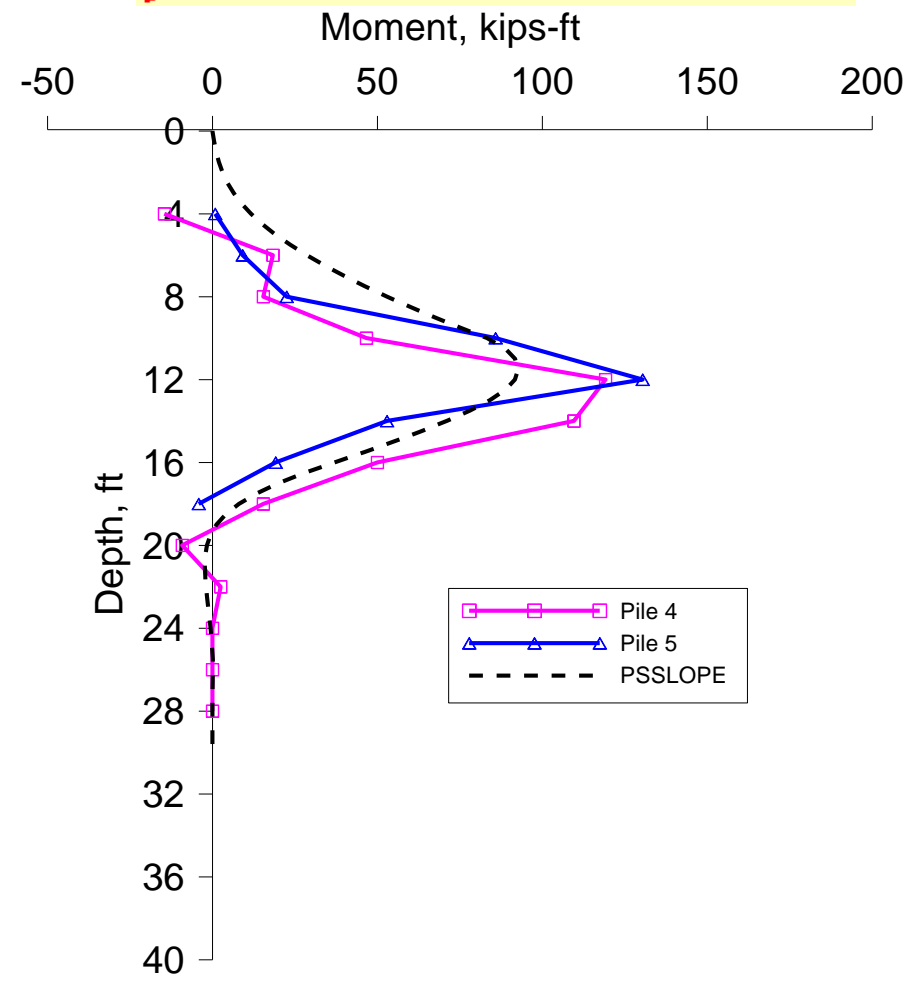
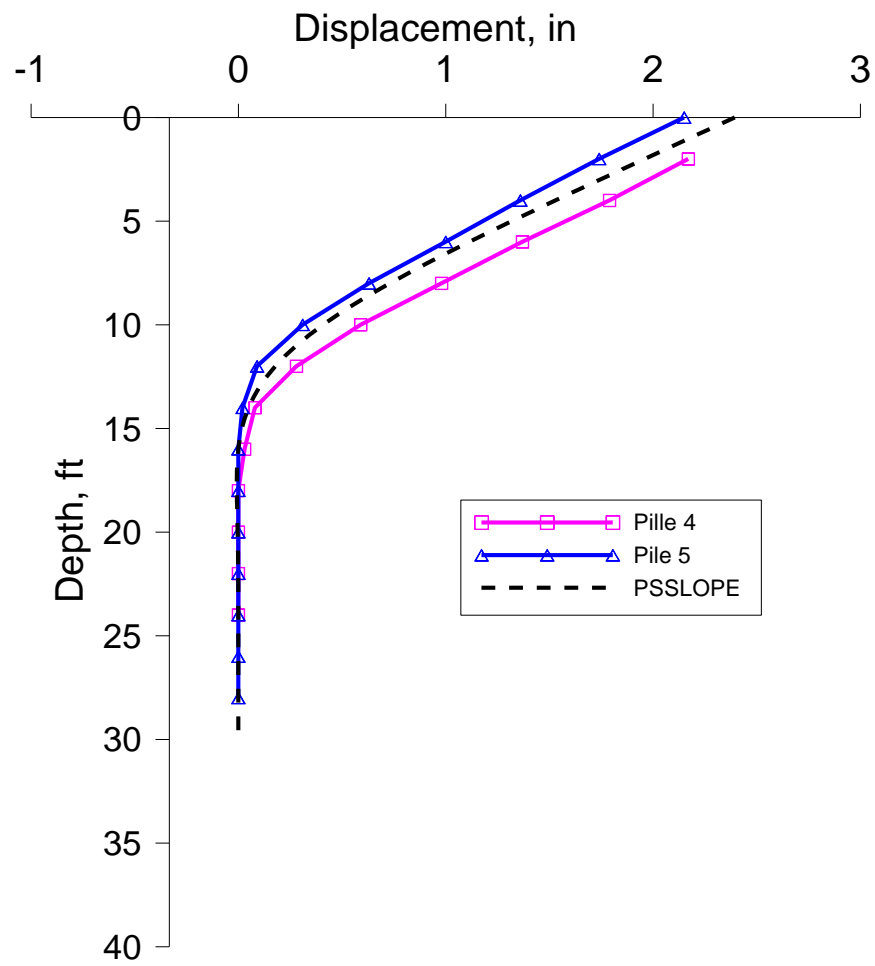
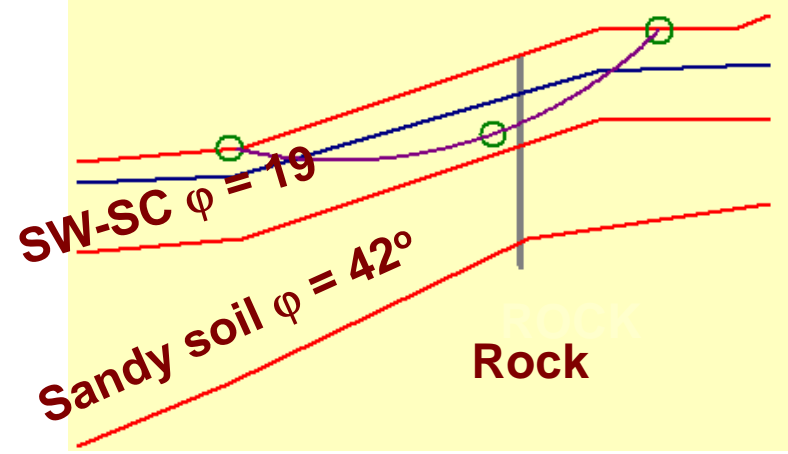
## Problem 1

## Problem 3



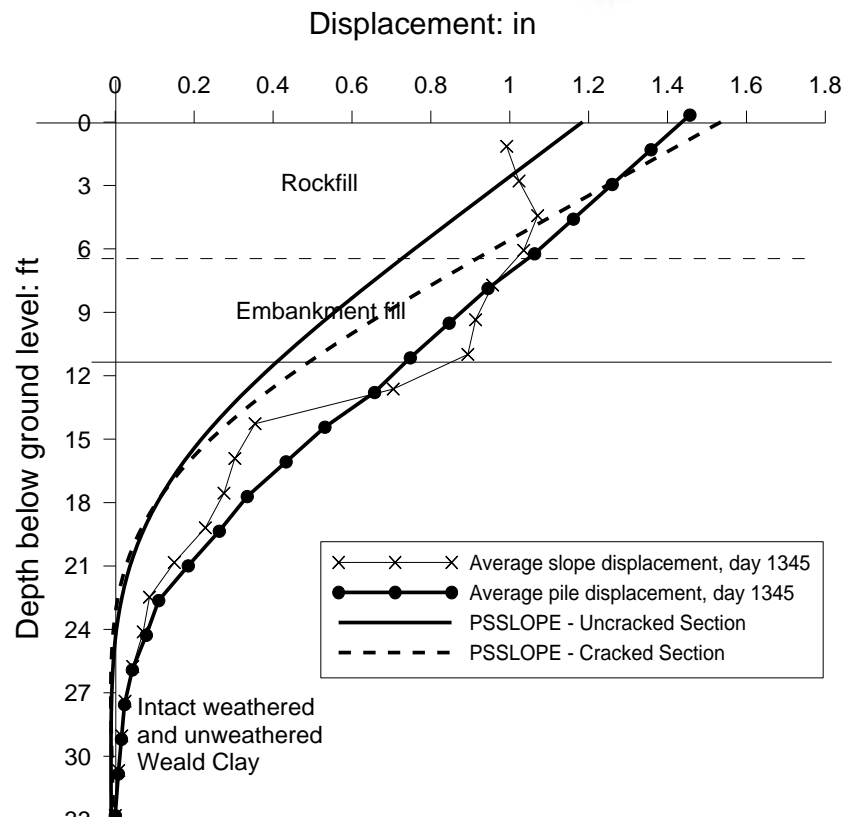
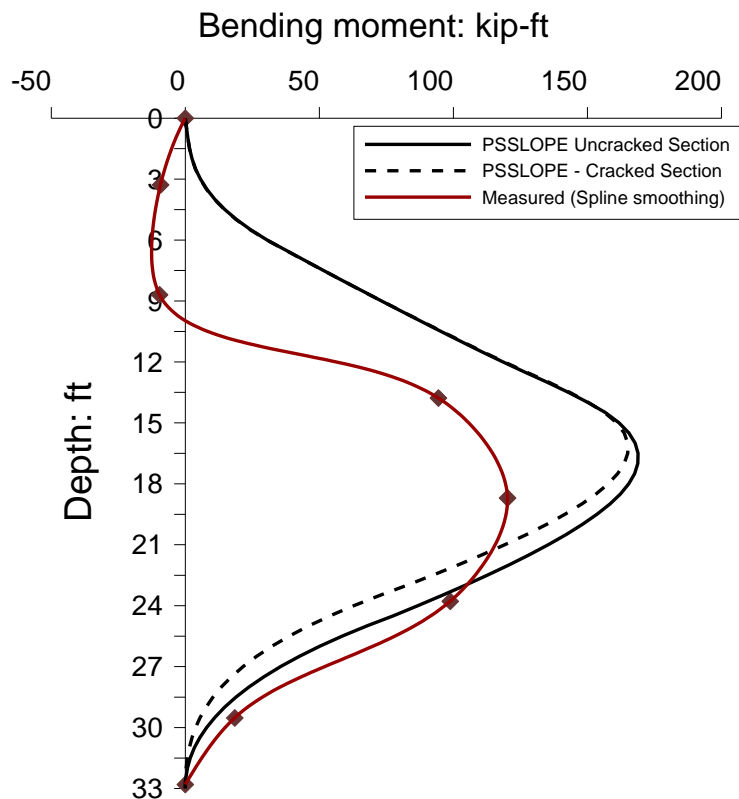
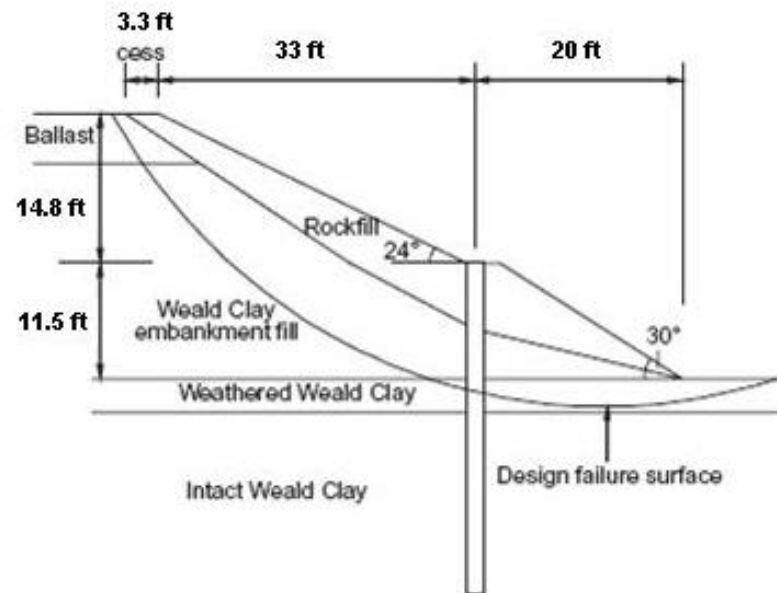
Unstable bedrock exerts a load on the pile above a plane/circle above the 1.5 factor of safety line. Analyze the loads on the pile from the earth loading and the loading from a retaining wall above.

# Tygart Lake Test Site, WV (After Richardson 2005)



# Embankment Profile, UK (Smethurst and Powerie 2007)

| Soil type                           | Unit weight,                  | Friction angle,   | Effective cohesion        |
|-------------------------------------|-------------------------------|-------------------|---------------------------|
|                                     | $\gamma$ : Ib/ft <sup>3</sup> | $\phi'$ : degrees | $c'$ : Ib/ft <sup>3</sup> |
| Weald Clay embankment fill          | 121                           | 25                | 20.9                      |
| Softened Weald Clay embankment fill | 121                           | 19                | 20.9                      |
| Weathered Weald Clay                | 121                           | 25                | 20.9                      |
| Weald Clay                          | 127                           | 30                | 104.4                     |
| Rockfill                            | 121                           | 35                | 0                         |



# **SUMMARY:**

**The current analysis/program provides the following:**

- Limit equilibrium analysis for existing or anticipated failure surface**
- Evaluation of the progressive driving pressure of sliding mass as a function of soil-pile displacement with varying safety factors**
- Consideration of the flow-around failure of soil which limits the soil mass interaction with the pile**
- The effect of pile properties and spacing**
- LRFD recommendations**
- Implementation of tie-back as an elastic support**